**APPENDIX H – CORRIDOR FEASIBILITY ASSESSMENT TRAFFIC ANALYSIS** 



**Transportation Impact Study** 

## Downtown Phoenix Active Transportation Corridor Feasibility Assessment

City of Phoenix

June 27, 2023 — TYLin Project #13328





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#### **APPENDIX A: SYNCHRO SUMMARY REPORTS**

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## 1 INTRODUCTION

## 1.1 Scope and Objective

TYLin was retained by the City of Phoenix to complete a Traffic Brief in support of the Active Transportation Corridor Assessment within the Downtown Phoenix area.

The purpose of this study is to determine existing traffic volumes and roadway capacity within the study area, during the weekday a.m. and p.m. peak periods, and to assess whether sufficient capacity exists to accommodate the addition of a proposed active transportation corridor on the following proposed alternative corridors/alignments:

- 1. An alignment following 1st Street in both directions through the study area;
- 2. An alignment following 3<sup>rd</sup> Street in both directions through the study area;
- 3. An alignment generally following 3<sup>rd</sup> Street through the study area, with the exception of the segment between Jefferson and Roosevelt Streets, through which the alignment follows 3<sup>rd</sup> Street southbound and 5<sup>th</sup> Street/4<sup>th</sup> Street northbound.

The three alignments listed above are illustrated in Figure 1-1.

This Traffic Brief consists of the following elements:

- A review of the existing study area transportation network;
- ► A summary of the expected traffic operations of the study area road network under existing (2022) conditions;
- An assessment of how capacity constraints on the study area road network may affect the feasibility of the proposed alternative alignments for the active transportation corridor.

A detailed scope was submitted by TYLin to the City of Phoenix ("the City") for review and comment.





CONNECT TO BIKE LANES NORTH OF ROOSEVELT MCDOWELL RD PAPAGO FWY ROOSEVELT ST FILLMORE ST WAN BUREN ST JEFFERSON ST LEGEND Study Boundary Corridor Option Study Intersection LINCOLN ST **T/Lin** RE-CONNECT TO 3RD ST TO CROSS FREEWAY

Figure 1-1 Proposed Active Transportation Corridor Alignments and Road Network





## **2 STUDY AREA**

## 2.1 Study Area Road Network

The study area being considered for the Phoenix Active Transportation Corridor is located within Downtown Phoenix. The study area is bounded by Interstate 10 to the north, East Lincoln Street to the south, North/South 5<sup>th</sup> Street to the east, and North/South 1<sup>st</sup> Street to the west. The study area, along with intersections under analysis in this traffic brief, is illustrated in **Figure 1-1**.

The following existing roads are included in the study area transportation network.

**1st Street** is a north-south minor collector roadway west of 3rd Street and east of Central Avenue. Within the study area, 1st street has an existing two-lane cross-section with a posted speed limit of 30mph. 1st Street has a three-lane cross-section, with two northbound lanes bounded between Adams Street and Jackson Street.

**3<sup>rd</sup> Street** is a north-south collector roadway west of 4<sup>th</sup>/5<sup>th</sup> Street and east of 1<sup>st</sup> Street. Within the study area, 3<sup>rd</sup> Street has an existing three-lane cross-section with two southbound lanes, and one northbound lane with a posted speed limit of 30mph. 3<sup>rd</sup> Street is also exclusively one-way southbound between a point just north of Roosevelt Street and Fillmore Street, as well as between Washington Street and a point just north of Buchanan Street.

**4<sup>th</sup> Street** is a northbound collector roadway bounded by Roosevelt Street and Fillmore Street. North of Roosevelt Street, its alignment defaults to 3<sup>rd</sup> Street; south of Fillmore Street, it defaults to 5<sup>th</sup> Street is west of 5<sup>th</sup> Street and east of 3<sup>rd</sup> Street. 4<sup>th</sup> Street has a three-lane cross-section and a posted speed limit of 30mph.

**5<sup>th</sup> Street** is a northbound collector roadway bounded by Fillmore Street and Jefferson Street. 5<sup>th</sup> Street has a three-lane cross-section and a posted speed limit of 30mph.

**Interstate 10 (I-10)** is an east-west interstate freeway with ten lanes, composed of four general traffic lanes and an HOV lane in each direction. The freeway has a posted speed limit of 65mph. The intersection of I-10 and 3<sup>rd</sup> Street provides an eastbound on-ramp and a westbound off-ramp to the I-10 HOV lanes.

**Roosevelt Street** is an east-west minor collector roadway south of I-10 and north of Fillmore Street. Within the study area, Roosevelt Street has an existing two-lane cross-section with a posted speed limit of 30mph.

**Fillmore Street** is an east-west collector roadway south of Roosevelt Street and north of Van Buren Street. Within the study area, Fillmore Street has an existing four-lane cross-section with a posted speed limit of 30mph.





**Van Buren Street** is an east-west arterial roadway south of Fillmore Street and north of Washington Street. Within the study area, Van Buren Street has an existing four-lane cross-section with a posted speed limit of 30mph.

**Washington Street** is a westbound arterial roadway south of Van Buren Street and north of Jefferson Street. Within the study area, Washington Street has an existing three-lane cross-section with a posted speed limit of 30mph.

**Jefferson Street** is an eastbound arterial roadway south of the Washington Street and north of Jackson Street. Within the study area, Jefferson Street has an existing four-lane cross-section with a posted speed limit of 30mph.

**Jackson Street** is an east-west minor collector roadway south of Jefferson Street and north of Lincoln Street. Within the study area, Jackson Street has an existing two-lane cross-section with a posted speed limit of 30mph.

**Lincoln Street** is an east-west collector roadway south of Jackson Street. Within the study area, Lincoln Street has an existing four-lane cross-section with a posted speed limit of 30mph.

Lane configuration diagrams for the existing study area road network are provided in **Figure 2-1** and **Figure 2-2**.

The following existing intersections have been included in the study area:

- ► 3rd Street & I-10 HOV (signalized)
- ► 1st Street & Roosevelt Street (signalized)
- 3rd Street & Roosevelt Street (signalized)
- ▶ 3rd Street/4th Street & Roosevelt Street (signalized)
- 1st Street & Fillmore Street (unsignalized)
- 3rd Street & Fillmore Street (signalized)
- ▶ 4th Street/5th Street & Fillmore Street (signalized)
- 1st Street & Van Buren Street (signalized)
- 3rd Street & Van Buren Street (signalized)
- 5th Street & Van Buren Street (signalized)
- ▶ 1st Street/1st Street & Washington Street (signalized)
- 3rd Street & Washington Street (signalized)
- ▶ 5th Street/5th Street & Washington Street (signalized)
- ► 1st Street & Jefferson Street (signalized)
- 3rd Street & Jefferson Street (signalized)





- ► 5th Street & Jefferson Street (signalized)
- ► 1st Street & Jackson Street (unsignalized)
- 3rd Street & Jackson Street (unsignalized)
- ► 1st Street & Lincoln Street (signalized)
- > 3rd Street & Lincoln Street (signalized)







Figure 2-1 Existing Road Network Lane Configuration – Van Buren to I-10

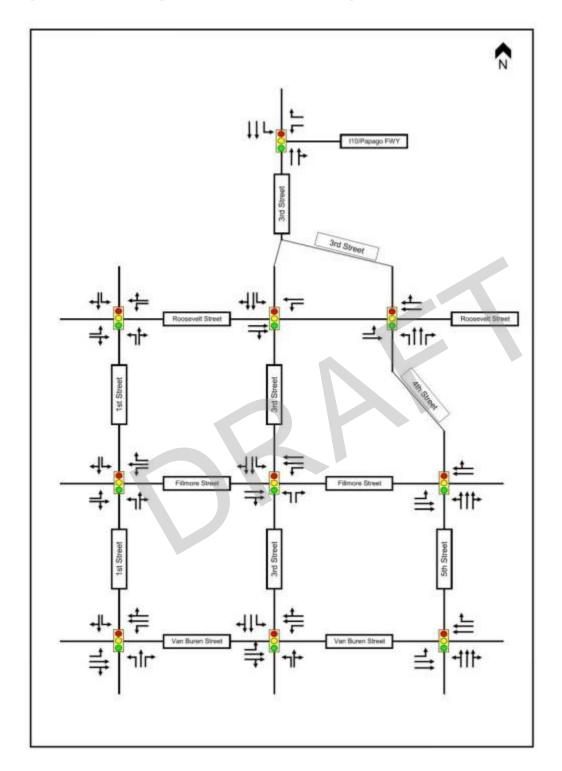
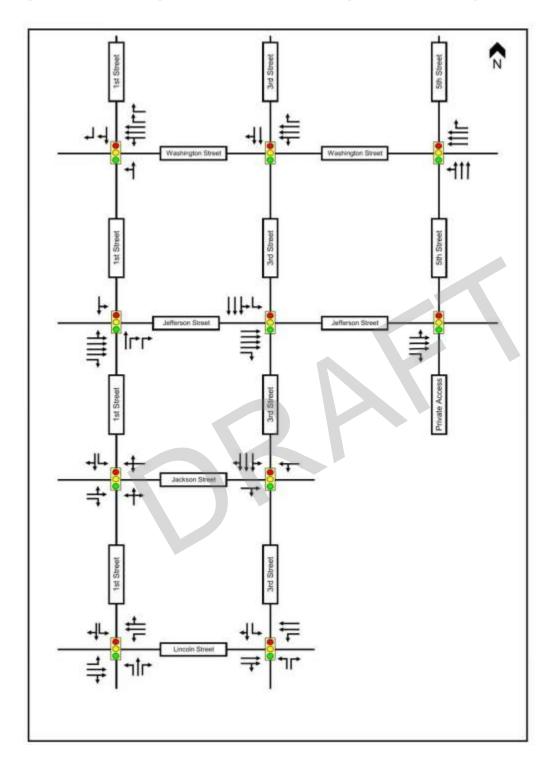






Figure 2-2 Existing Road Network Lane Configuration – Washington to Lincoln





## 2.2 Study Area Transit Network

The study area is well-served by the existing Valley Metro transit network, including bus routes as well as the Valley Metro Rail light rail transit (LRT) line.

LRT service within the study area is provided via the Valley Metro Rail line, which proceeds eastwest (eastbound via Jefferson Street and westbound via Washington Street) before turning to a north-south alignment along the study area's western boundary (northbound via Central Avenue and southbound via 1st Avenue, before merging to bidirectional service via Central Avenue north of Roosevelt Street). Stops within or adjacent to the study area are provided at Roosevelt Street and Central Avenue/1st Avenue, Van Buren Street and Central Avenue/1st Avenue, Jefferson Street/1st Avenue and Washington Street/Central Avenue, 3rd Street and Washington Street/Jefferson Street.

Bus service within or adjacent to the study area is provided on corridors including Roosevelt Street, Fillmore Street, Van Buren Street, Adams Street, 1<sup>st</sup> Avenue, Central Avenue, 3<sup>rd</sup> Street, 4<sup>th</sup> Street, 5<sup>th</sup> Street, 7<sup>th</sup> Street, and Interstate-10.

The existing transit network in the study area is illustrated in Figure 2-3.

## 2.3 Study Area Active Transportation Network

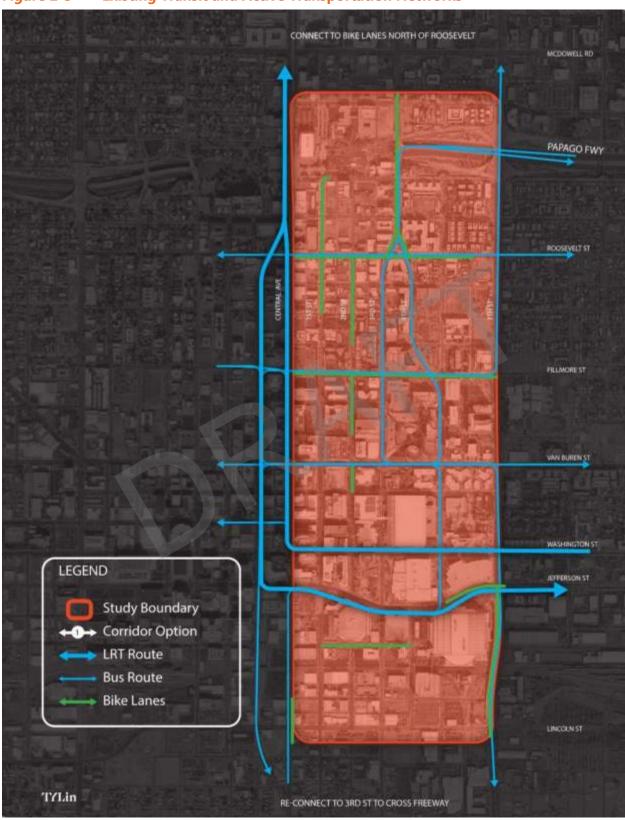
The study area features numerous active transportation connections. Most streets within the study area boundaries feature sidewalks, of varying widths, on both sides of the road. In conjunction with the extensive network of off-street pathways and trails within the study area, both within municipal parks/plazas and on publicly accessible private properties, the study area features a fine-grained pedestrian network providing access to the many major trip generators located within its boundaries, including Footprint Center, Chase Field, Phoenix Convention Center, Arizona Science Center, Arizona Center, Symphony Hall, and Arizona State University.

Cycling infrastructure is present on many streets within the study area; however, it presently takes the form of a discontinuous patchwork of cycling lanes that offers limited opportunities for continuous north-south or east-west travel through the study area. Painted and/or separated cycling lanes are present on segments of Central Avenue, 1<sup>st</sup> Street, 2<sup>nd</sup> Street, 3<sup>rd</sup> Street, 4<sup>th</sup> Street, 7<sup>th</sup> Street, Jackson Street, Jefferson Street, Fillmore Street, and Roosevelt Street. In addition, many study area roadways feature painted 'sharrows' to indicate cycling routes.

The existing cycling network in the study area is also illustrated in **Figure 2-3**.







**Figure 2-3 Existing Transit and Active Transportation Networks** 





## 3 TRAFFIC CAPACITY ANALYSIS

Traffic capacity analysis identifies how well the study area intersections are operating under existing traffic conditions. The purpose of this analysis is to assess where, if any, capacity constraints on the study area road network may exist and how they may affect the proposed alternative alignments for the north-south active transportation corridor. The analysis contained in this report utilized the Highway Capacity Manual (HCM) 2000 techniques within the Synchro software package. The reported intersection volume-to-capacity ratios (v/c) are a measure of the saturation volume for each turning movement, while the levels-of-service (LOS) are a measure of the average delay (in seconds) for each turning movement. Queueing characteristics are reported as the predicted 95th percentile queue for each turning movement.

For all intersections and traffic movements within the study area, critical intersections and movements shall be highlighted (**in bold**). 'Critical' intersections and movements include:

- v/c ratios of or exceeding 0.90 for overall intersection operations, through movements, or shared through/turning movements;
- v/c ratios of or exceeding 1.00 for exclusive movements;
- Any movements or overall intersection operations experiencing level of service 'E' or "F'.

The Synchro model used for this analysis was based on an existing provided by the City of Phoenix, as originally prepared by Burgess & Niple consultants for the 2020 Downtown Transportation Plan Update. The model was updated with multi-modal turning movement counts conducted by the United Civil Group sub-consultants for the study area intersections on Tuesday, October 11, 2022.

For the future conditions, in addition to considering the two options for 3<sup>rd</sup> Street, the 2020 Sunburst event access traffic control plans for less than 25,000, and over 25,000 were also considered for each of the scenarios.

Detailed Synchro summary reports are provided in **Appendix A.** 

### 3.1 Existing Conditions

Synchro traffic capacity analysis results for the study area intersections are summarized in **Table 3-1** for both the weekday a.m. and p.m. peak hours under existing (2022) traffic conditions.





**Table 3-1 Existing 2022 Capacity Analysis** 

Table 5-1		2022 Cap		/eekday AN	A Doole Ho	ur	14	/eekday PN	/ Doals Ho	ır
		Storage	V		и геак по	ur 95%	V		л геак по	ur 95%
Intersection	Movement	Length (ft)	V/C	Delay (s)	LOS	Queue (ft)	V/C	Delay (s)	LOS	Queue (ft)
	Overall	-	0.3	8	Α	-	0.32	15	В	-
	EBL	173	0.03	3	Α	1 veh	0.08	12	В	1 veh
	EBTR	-	0.3	4	Α	75	0.36	14	В	146
	WBL	98	0.19	5	Α	28	0.08	12	В	23
1st St & Van Buren St	WBTR	-	0.22	4	Α	55	0.43	15	В	186
(signalized)	NBL	112	0.27	28	С	48	0.17	19	В	60
	NBT	-	0.17	27	С	45	0.13	18	В	68
	NBR	112	0.02	27	C	1 veh	0.05	17	В	24
	SBL	95	0.14	27	С	28	0.1	18	В	37
	SBTR	-	0.29	28	C	65	0.1	18	В	47
	Overall	-	0.27	10	В		0.31	9	Α	-
	EBL	82	0.05	5	Α	1 veh	0.05	4	Α	1 veh
	EBTR	-	0.31	8	Α	103	0.34	7	Α	136
3rd St & Van	WBL	46	0.05	5	А	1 veh	0.08	1	А	1 veh
Buren St	WBTR	=	0.27	8	А	90	0.31	4	Α	149
(signalized)	NBL	151	0	24	C	1 veh	0.06	32	C	1 veh
	NBTR	1	0.02	24	С	1 veh	0.06	32	С	31
	SBL	144	0.13	25	С	36	0.17	32	С	46
	SBTR	-	0.18	25	С	44	0.23	33	С	55
	Overall	-	0.11	22	С	-	0.12	18	В	-
	EBTR		0.04	4	Α	1 veh	0.08	4	Α	22
	WBL	158	0.02	3	Α	1 veh	0.03	2	Α	1 veh
3rd St & Fillmore St	WBT	-	0.03	3	Α	1 veh	0.05	2	Α	1 veh
(signalized)	NBL	-	0.05	32	С	1 veh	0.13	32	С	34
, ,	NBR	-	0.02	31	С	24	0.05	32	С	37
	SBL	-	0.24	33	С	73	0.09	32	С	34
	SBTR	-	0.41	34	С	101	0.31	33	С	78
	Overall	-	0.18	18	В	-	0.35	18	В	-
	EBL	-	0.17	6	Α	1 veh	0.25	6	Α	1 veh
4th St &	EBT	-	0.6	10	В	30	0.71	14	В	41
Roosevelt St (signalized)	WBTR	98	0.39	31	С	98	0.42	28	С	119
(= 9 =====)	NBL	-	0.02	8	Α	1 veh	0.07	12	В	48
	NBT	-	0.04	8	Α	25	0.17	13	В	94





			W	/eekday Al	И Peak Ho	ur	W	/eekday PN	И Peak Ho	ur
Intersection	Movement	Storage Length (ft)	V/C	Delay (s)	LOS	95% Queue (ft)	V/C	Delay (s)	LOS	95% Queue (ft)
	NBR	-	0	8	Α	0	0	11	В	0
	Overall	-	0.26	18	В	-	0.33	24	С	-
	EBTR	-	0.41	33	С	102	0.47	29	С	131
3rd St & Roosevelt St	WBL	120	0.15	11	В	1 veh	0.17	18	В	1 veh
(signalized)	WBT	-	0.66	17	В	41	0.79	30	С	84
	SBL	-	0.02	7	Α	1 veh	0.04	9	Α	32
	SBTR	-	0.15	7	Α	63	0.14	10	А	62
	Overall	-	0.28	19	В	-	0.4	38	D	-
	WBL	190	0.3	19	В	110	0.17	18	В	68
3rd St & I-10 HOV	WBR	-	0.14	18	В	44	0.06	17	В	28
(signalized)	NBTR	-	0.17	17	В	53	0.38	19	В	115
	SBL	121	0.41	36	D	82	1.12	132	F	267
	SBT	-	0.21	17	В	72	0.3	18	В	100
	Overall	-	0.14	25	С	-	0.16	8	Α	-
	EBTR	-	1.50dr	26	С	98	2.20dl	1	Α	43
	WBL	105	0.04	24	С	1 veh	0.01	1	Α	1 veh
3rd St & Lincoln St	WBT	-	0.39	27	С	102	0.08	1	Α	1 veh
(signalized)	NBL	1	0	7	А	1 veh	0.19	43	D	28
	NBR	157	0.01	7	Α	1 veh	0.01	42	D	1 veh
	SBL	144	0.01	7	Α	1 veh	0.25	43	D	41
	SBTR	-	0.02	7	Α	1 veh	0.29	43	D	56
	Overall	-	0.24	7	Α	-	0.27	8	Α	-
	EBL	131	0.12	3	А	1 veh	0.12	4	Α	1 veh
5th St & Van Buren St	EBTR	-	0.21	3	Α	58	0.29	4	Α	81
(signalized)	WBT	-	0.26	6	Α	102	0.28	8	Α	112
	WBR	207	0.12	6	Α	1 veh	0.05	6	Α	1 veh
	NBLTR	-	0.11	34	С	24	0.12	32	C	33
	Overall	-	0.1	12	В	-	0.2	23	С	-
5th St/4th St & Fillmore St (signalized)	EBL	128	0.03	8	Α	1 veh	0.09	5	Α	37
	EBT	-	0.05	8	Α	31	0.09	5	Α	41
	WBTR	-	0.09	4	А	25	0.06	4	А	1 veh
	NBLTR	-	0.12	32	С	32	0.64	36	D	112
	Overall	-	0.24	5	Α	-	0.18	6	Α	-
	WBT	-	0.25	2	А	51	0.17	1	Α	1 veh





		(ft) V/C   Delay   LOS   Queue   V/C   Delay   LO							Л Peak Ho	ur
Intersection	Movement	Length	V/C	Delay (s)	LOS		V/C	Delay (s)	LOS	95% Queue (ft)
5th St &	WBR	295	0.04	2	Α	m11	0.04	1	Α	m5
Washington St (signalized)	NBTL	-	0.2	53	D	32	0.26	54	D	40
5th St &	Overall	-	0.09	1	Α	-	0.29	2	Α	-
Jefferson St	EBTL	-	0.09	1	Α	40	0.29	2	Α	132
(signalized)	EBR	-	0	0	0	0	0	0	0	0
	Overall	-	0.15	12	В	-	0.31	11	В	-
	EBTL	-	0.08	3	А	1 veh	0.21	6	Α	61
1st St & Jefferson St	EBR	144	0.04	3	Α	1 veh	0.06	5	Α	28
(signalized)	NBT	112	0.47	24	С	80	0.26	18	В	57
	NBR	-	0.17	21	С	25	0.38	19	В	50
	SBTL	-	0.44	24	С	67	0.57	22	С	92
	Overall	-	0.29	15	В	-	0.34	16	В	-
	WBTL	-	0.18	5	Α	64	0.23	10	Α	75
Washington	WBR	161	0.04	4	А	1 veh	0.04	8	А	1 veh
St & 1st St (signalized)	NBTL	-	0.7	39	D	155	0.52	27	С	177
, ,	SBTR	-	0.25	29	С	72	0.22	21	С	92
	SBR	115	0.28	30	С	67	0.13	21	С	46
3rd St &	Overall	-	0.26	26	С	-	0.22	28	С	-
Washington	WBTL	-	0.63	29	С	188	0.63	32	С	158
St	WBR	210	0	23	С	1 veh	0.08	27	С	32
(signalized)	SBTR	-	0.08	7	Α	37	0.07	5	Α	28
	Overall	-	0.19	10	В	-	0.31	26	С	-
	EBL	89	0.07	7	Α	23	0.16	22	С	37
	EBTR	-	0.24	8	Α	98	0.71	33	С	327
1st St &	WBL	92	0.08	7	Α	26	0.31	27	С	53
Roosevelt St	WBTR	1	0.22	8	Α	93	0.56	28	С	253
(signalized)	NBL	115	0.05	24	С	26	0.04	9	Α	1 veh
	NBTR	-	0.03	23	С	25	0.07	9	Α	29
	SBL	112	0.02	23	С	1 veh	0.04	9	Α	1 veh
	SBTR	-	0.07	24	С	37	0.04	9	Α	1 veh
3rd St &	Overall	-	0.08	9	Α	-	0.21	10	В	-
Jefferson St	EBT	-	0.07	2	Α	1 veh	0.29	10	Α	77
(signalized)	EBR	164	0.02	2	Α	1 veh	0.11	9	Α	38





		Storage	W	eekday AN	Л Peak Ho	ur	W	/eekday PN	Л Peak Ho	ur
Intersection	Movement	Length (ft)	V/C	Delay (s)	LOS	95% Queue (ft)	V/C	Delay (s)	LOS	95% Queue (ft)
	SBL	141	0.02	27	С	26	0.03	15	В	23
	SBTL	-	0.23	28	С	28	0.08	15	В	25
3rd Street &	EBTR	-	-	-	-	-	-	-	-	-
Jackson Street	WBLT	-	-	-	-	-	-	-	-	-
(unsignalized)	SBLTR	-	-	-	-	-	-	-	-	-
1st Stroot &	EBLTR	-	0.1	8	Α	-	0.28	10	А	-
1st Street & Jackson Street (unsignalized)	WBLTR	-	0.14	8	Α	-	0.13	8	Α	-
	NBLTR	-	0.25	9	Α	-	0.17	9	Α	-
	SBLTR	-	0.09	7	Α	-	0.18	8	Α	-
	EBLTR	-	0.06	3	Α	1 veh	0.03	2	Α	1 veh
1st Street &	WBLTR	-	<0.01	0	Α	0	<0.01	0	А	0
Lincoln Street (unsignalized)	NBLTR	-	0.09	15	С	1 veh	0.09	14	В	1 veh
, ,	SBLTR	-	0.11	13	В	1 veh	0.31	16	С	33
1st Street & Fillmore Street (unsignalized)	EBLTR	98	-	-	-	-	-	-	-	-
	WBLTR	89	-	-	-	-	-	-	-	-
	NBLTR	98		-	-	-	=	-	-	-
(unsignalized)	SBLTR	98	-	-	-	-	-	-	-	-

Under existing conditions, all study area intersections are operating with overall reserve capacity and acceptable delays during both the weekday a.m. and p.m. peak hours. Individual movements and overall intersections operate with LOS D or better during the peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3rd Street, i.e. movements to the freeway ramp. It should be noted that, based on Google Street View imagery, at some point between March 2022 and January 2023 this intersection was reconstructed to add separated cycling lanes. As part of this reconstruction, the southbound left-turn lane was lengthened (from a storage length of approximately 180ft to approximately 260ft). Along with other potential mitigation measures related to traffic signal timings, this may mitigate the operational issues noted for this movement. Since it cannot be verified that these changes were in place at the time of turning movement count collection in October 2022 and for the sake of consistency, the conservative approach of not implementing these changes in the Synchro model was followed for this study.

Volume-to-capacity ratios at all study area intersections and movements are within the acceptable range with the exception of the eastbound through-right movement at the intersection of 3rd Street and Lincoln Street, which experiences a volume-to-capacity ratio of 1.50 and 2.20 in the a.m. and p.m. respectively. The LOS and delay for this movement, however, are well within acceptable limits in both time periods.





Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during both the weekday a.m. and p.m. peak hours under existing conditions. The only exception is the southbound left movement at the intersection of I-10 and 3rd Street, with a queue of 267ft, as compared to a storage length of 180ft; however, as discussed above, the left-turn lane storage length for this movement has since been lengthened to approximately 260ft, mitigating this operational issue. The westbound right movement at the intersection of 5<sup>th</sup> Street and Washington Street experiences a metered queue, which operate within limits.

#### 3.2 Future Conditions

The Study Area was assessed with three possible configurations for N 3<sup>rd</sup> Street within the study area. Option 1 and 3 of the possible configurations had a single lane going north and south respectively, with bicycles on either side of the street in Option 1 and with the lanes on the same side of the street with Option 3. The three lane configurations can be seen in **Figure 3-1**, **Figure 3-2**, and **Figure 3-3** respectively.

Figure 3-1 Future Conditions Option 1

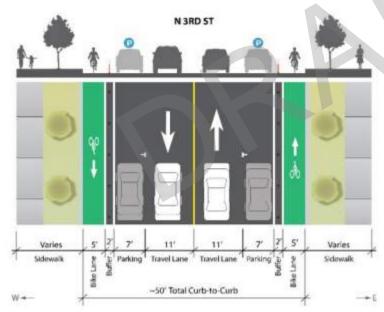
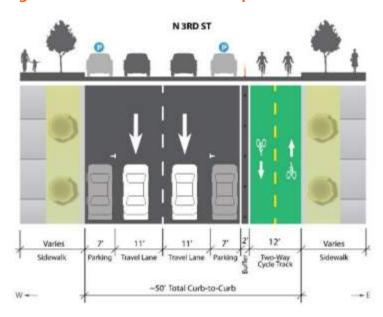


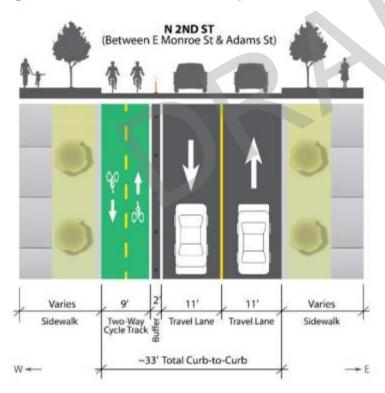




Figure 3-2 Future Conditions Option 2



**Figure 3-3 Future Conditions Option 3** 



For the sake of traffic analysis, both are functionally the same, and were treated as the same future conditions and will henceforth be referred to as Option 1 for simplification. Option 2 of the possible future conditions had N 3<sup>rd</sup> Street eliminating the northbound movement completely and considering 2 lanes going southbound along the corridor.





Additionally, the two scenarios in addition to their base future configurations, also considered the Sunburst Traffic Control Plan's two modifications for each of the two options. The Traffic Control Plan considered the standard egress of under 25,000, and over 25,000.

The scenarios studied have can be seen below:

#### Option 1 – Bidirectional along N 3<sup>rd</sup> St

- 1-0 Without any event
- 1-1 With <25,000 Event
- 1-2 With 25,000+ Event

#### Option 2 – Unidirectional along N 3<sup>rd</sup> St (SB)

- 2-0 Without any event
- 2-1 With <25,000 Event
- 2-2 With 25,000+ Event

#### 3.2.1 Capacity Analysis for Options 1

#### 3.2.1.1 Option 1 – AM Traffic Capacity Analysis

The traffic capacity analysis for the AM peak hour for the three Option 1 configurations are shown in **Table 3-2** below.

**Table 3-2 Option 1-0 to 1-2 AM Traffic Capacity Analysis** 

		Storage	Optio	n 1-0 Wee	ekday AM our	Peak	Optio	on 1-1 Wee Ho	ekday AM our	Peak	Option 1-2 Weekday AM Peak Hour			
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
						Signa	lized							
	Overall	-	0.30	8	Α	-	0.30	8	Α	-	0.30	8	Α	-
	EBL	173	0.03	3	Α	8	0.03	3	Α	8	0.03	3	Α	8
	EBTR	-	0.30	4	Α	75	0.30	4	Α	75	0.30	4	Α	75
	WBL	98	0.19	5	Α	28	0.19	5	Α	28	0.19	5	Α	28
1st St & Van Buren	WBTR	-	0.22	4	Α	55	0.22	4	Α	55	0.22	4	Α	55
St	NBL	112	0.27	28	С	48	0.27	28	С	48	0.27	28	С	48
	NBT	ı	0.17	27	С	45	0.17	27	С	45	0.17	27	С	45
	NBR	112	0.02	27	С	18	0.02	27	С	18	0.02	27	С	18
	SBL	95	0.14	27	С	28	0.14	27	С	28	0.14	27	С	28
	SBTR	-	0.29	28	С	65	0.29	28	С	65	0.29	28	С	65
	Overall	-	0.32	11	В	-	0.32	11	В	-	0.32	11	В	1





		Storage	Optio	n 1-0 Wee	ekday AM our	Peak	Optio	n 1-1 We	ekday AM our	Peak	Optio	n 1-2 We	ekday AM our	Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	EBL	82	0.05	5	Α	9	0.05	5	Α	9	0.05	5	Α	9
	EBTR	-	0.31	8	Α	105	0.31	8	Α	105	0.31	8	Α	105
	WBL	46	0.05	5	Α	9	0.05	5	Α	9	0.05	5	Α	9
3rd St & Van Buren	WBTR	-	0.27	8	Α	91	0.27	8	Α	91	0.27	8	Α	91
St	NBL	151	0.00	24	С	4	0.00	24	С	4	0.00	24	С	4
	NBTR	ı	0.02	24	С	16	0.02	24	С	16	0.02	24	С	16
	SBL	144	0.13	25	С	36	0.13	25	С	36	0.13	25	С	36
	SBTR	1	0.40	26	С	103	0.40	26	С	103	0.40	26	С	103
	Overall	-	0.19	25	С	-	0.19	25	С	-	0.19	25	С	-
	EBTR	-	0.05	4	Α	21	0.05	4	Α	21	0.05	4	Α	21
	WBL	158	0.02	4	Α	11	0.02	4	Α	11	0.02	4	Α	11
3rd St &	WBT	-	0.04	4	Α	16	0.04	4	A	16	0.04	4	Α	16
Fillmore St	NBL	-	0.06	29	С	15	0.06	29	С	15	0.06	29	С	15
	NBTR	-	0.00	45	D	0	0.00	45	D	0	0.00	45	D	0
	SBL	-	0.20	30	С	63	0.20	30	С	63	0.20	30	С	63
	SBTR	-	0.71	39	D	204	0.71	39	D	204	0.71	39	D	204
	Overall	-	0.18	18	В	-	0.18	18	В	-	0.18	18	В	-
	EBL	-	0.17	5	Α	7	0.17	5	Α	7	0.17	5	Α	7
4th St &	EBT	-	0.60	10	Α	31	0.60	10	Α	31	0.60	10	Α	31
Roosevelt	WBTR	98	0.39	31	С	98	0.39	31	С	98	0.39	31	С	98
St	NBL	-	0.02	8	Α	17	0.02	8	Α	17	0.02	8	Α	17
	NBT	-	0.04	8	Α	25	0.04	8	Α	25	0.04	8	Α	25
	NBR	-	0.00	8	Α	0	0.00	8	Α	0	0.00	8	Α	0
	Overall	-	0.27	18	В	-	0.27	18	В	-	0.27	18	В	-
	EBTR	-	0.41	33	С	102	0.41	33	С	102	0.41	33	С	102
3rd St &	WBL	120	0.15	11	В	9	0.15	11	В	9	0.15	11	В	9
Roosevelt	WBT	-	0.66	17	В	41	0.66	17	В	41	0.66	17	В	41
St	NBL	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	NBR	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	SBLTR		0.16	7	Α	68	0.16	7	Α	68	0.16	7	Α	68
	Overall	-	0.28	19	В	-	0.28	19	В	-	0.28	19	В	-
	WBL	190	0.30	19	В	110	0.30	19	В	110	0.30	19	В	110
3rd St & I10	WBR	-	0.14	18	В	44	0.14	18	В	44	0.14	18	В	44
HOV	NBTR	-	0.17	17	В	53	0.17	17	В	53	0.17	17	В	53
	SBL	121	0.41	36	D	82	0.41	36	D	82	0.41	36	D	82
	SBT	-	0.21	17	В	72	0.21	17	В	72	0.21	17	В	72





		Storage	Optio	n 1-0 Wee Ho	ekday AM our		Optio	n 1-1 We	ekday AM our		Optio	on 1-2 We	ekday AM our	
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	Overall	-	0.14	33	С	-	0.03	0	Α	-	0.03	0	Α	-
	EBTR	-	0.47	34	С	107	0.00	0	0	-	0.00	0	0	-
	WBL	105	0.06	31	С	16	0.00	0	0	-	0.00	0	0	-
3rd St &	WBT	-	0.56	35	С	132	0.00	0	0	-	0.00	0	0	-
Lincoln St	NBL	-	0.00	4	Α	4	0.00	0	0	-	0.00	0	0	-
	NBTR	157	0.00	47	D	0	0.00	0	0	-	0.00	0	0	-
	SBL	144	0.01	4	Α	10	0.00	0	0	-	0.00	0	0	-
	SBTR	-	0.02	4	Α	9	0.03	0	Α	0	0.03	0	Α	0
	Overall	-	0.24	7	Α	-	0.24	7	Α	-	0.24	7	Α	-
	EBL	131	0.12	3	Α	18	0.12	3	Α	18	0.12	3	Α	18
5th St &	EBTR	-	0.21	3	Α	58	0.21	3	Α	58	0.21	3	Α	58
Van Buren St	WBT	-	0.26	6	Α	102	0.26	6	Α	102	0.26	6	Α	102
	WBR	207	0.12	6	Α	20	0.12	6	Α	20	0.12	6	Α	20
	NBLTR	-	0.11	34	С	24	0.11	34	С	24	0.11	34	С	24
	Overall	-	0.10	12	В		0.10	12	В	-	0.10	12	В	-
5th St/4th St & Fillmore St	EBL	128	0.03	7	Α	15	0.03	7	Α	15	0.03	7	Α	15
	EBT	-	0.05	6	A	24	0.05	6	Α	24	0.05	6	Α	24
	WBTR	-	0.09	4	Α	25	0.09	4	Α	25	0.09	4	Α	25
	NBLTR	-	0.12	32	С	32	0.12	32	С	32	0.12	32	С	32
	Overall	-	0.24	5	Α		0.24	5	Α	-	0.24	5	Α	-
	WBT	-	0.25	2	Α	51	0.25	2	Α	51	0.25	2	Α	51
Washington St	WBR	295	0.04	2	Α	m11	0.04	2	Α	m11	0.04	2	Α	m11
	NBTL	-	0.20	53	D	32	0.20	53	D	32	0.20	53	D	32
5th St &	Overall	-	0.09	1	Α	-	0.09	1	Α	-	0.09	1	Α	-
Jefferson	EBTL	-	0.09	1	Α	40	0.10	1	Α	46	0.10	1	Α	46
St	EBR	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.15	12	В	-	0.07	0	Α	-	0.17	0	Α	-
	EBTL	-	0.08	3	Α	20	0.06	0	Α	0	0.14	0	Α	0
1st St &	EBR	144	0.04	3	Α	16	0.00	0	0	-	-	-	-	-
Jefferson St	NBT	112	0.47	24	С	80	0.00	0	0	-	0.00	0	0	-
	NBR	-	0.17	21	С	25	0.00	0	0	-	0.00	0	0	-
	SBTL	-	0.44	24	С	67	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.29	15	В	-	0.31	11	В	-	0.35	18	В	-
Washington	WBTL	-	0.18	5	Α	64	0.24	5	Α	90	0.25	6	Α	103
St & 1st St	WBR	161	0.04	4	Α	15	0.04	4	Α	14	0.04	4	Α	17
	NBL	-								-	0.11	28	С	44





		Storage	Optio	n 1-0 Wee	ekday AM our	l Peak	Optio	n 1-1 We	ekday AM our	l Peak	Optio	on 1-2 We	ekday AM our	l Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	NBTL	-	0.70	39	D	155	0.00	0	0	-	0.69	36	D	194
	SBTR	ı	0.25	29	С	72				-				-
	SBR	115	0.28	30	С	67	0.62	37	D	128	0.54	32	С	120
	Overall	-	0.31	26	С	-	0.29	26	С	-	0.29	26	С	-
3rd St &	WBTL	-	0.64	28	С	191	0.70	26	С	258	0.70	26	С	258
Washington	WBR	210	0.00	22	С	5	0.00	18	В	4	0.00	18	В	4
St	NBTL	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	SBTR	-	0.16	8	Α	79	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.19	10	В	-	0.19	10	В	-	0.19	10	В	-
	EBL	89	0.07	7	Α	23	0.07	7	Α	23	0.07	7	Α	23
	EBTR	-	0.24	8	Α	98	0.24	8	Α	98	0.24	8	Α	98
1st St &	WBL	92	0.08	7	Α	26	0.08	7	A	26	0.08	7	Α	26
Roosevelt	WBTR	-	0.22	8	Α	93	0.22	8	Α	93	0.22	8	Α	93
St	NBL	115	0.05	24	С	26	0.05	24	С	26	0.05	24	С	26
	NBTR	-	0.03	23	С	25	0.03	23	С	25	0.03	23	С	25
-	SBL	112	0.02	23	С	15	0.02	23	С	15	0.02	23	С	15
	SBTR	-	0.07	24	С	37	0.07	24	С	37	0.07	24	С	37
	Overall	-	0.12	10	Α	-	0.06	0	Α	-	0.06	0	Α	-
	EBT	-	0.07	2	A	16	0.06	0	Α	0	0.06	0	Α	0
3rd St &	EBR	164	0.02	2	Α	8				-				-
Jefferson St	NBTR	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	SBL	141	0.23	26	C	41	0.00	0	0	-	0.00	0	0	-
	SBT	-	0.44	28	С	71	0.00	0	0	-	0.00	0	0	-
						Unsign	alized							
Jud	EBTR	-	0.07	7	Α	-	0.06	7	Α	-	0.06	7	Α	-
3rd Street &	WBLT	-	0.03	7	Α	-	0.03	7	Α	-	0.03	7	Α	-
Jackson	NBLTR	-	0.00	7	Α	-	0.00	7	Α	-	0.00	7	Α	-
Street	SBLTR	-	0.11	7	Α	-	0.11	7	Α	-	0.11	7	Α	-
	EBLTR	-	0.10	8	Α	-	0.10	8	Α	-	0.10	8	Α	-
1st Street	WBLTR	-	0.14	8	Α	-	0.15	9	Α	-	0.15	9	Α	-
&	NBLTR	-	0.25	9	Α	_	0.25	9	Α	_	0.25	9	Α	-
Jackson Street	SBL	-	0.03	8	Α	-	0.03	8	Α	-	0.03	8	Α	-
	SBTR	-	0.09	7	-	-	0.09	7	-	-	0.09	7	-	-
1st Street	EBLT	-	0.06	3	Α	5	0.28	8	Α	29	0.28	8	Α	29
& Lincoln	EBTR	-	0.10	0	-	-	-	-	-	-	-	-	-	-
Street	WBLT	-	0.00	0	Α	-	0.00	0	Α	0	0.00	0	Α	0





		Storage						n 1-1 Wee	ekday AM our	Peak	Option 1-2 Weekday AM Peak Hour			
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	WBTR	-	0.14	0	-	-	0.14	0	-	0	0.14	0	-	0
	NBLTR	-	0.08	15	С	7	0.22	35	D	20	0.22	35	D	20
	SBLTR	-	0.10	13	В	8	0.21	24	С	19	0.21	24	С	19
1st Street	EBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
%	WBLTR	89	-	-	-	-	-	-	-	-	-	-	-	-
Fillmore Street	NBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
	SBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-

#### 1-0 AM – Future conditions without any events

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday a.m. peak hours. Individual movements and over intersections operate with LOS D or better during the a.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the a.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday a.m. peak hour under future conditions for option 1-0.

#### 1-1 AM – Future conditions with <25,000 Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday a.m. and p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the a.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the a.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday a.m. peak hour under future conditions for option 1-1. The only exception is the southbound right movement at the intersection of Washington Street and 1<sup>st</sup> Street, with a queue of 128ft, as compared to the storage length of 115ft.

#### 1-2 AM - Future conditions with 25,000+ Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during





both the weekday a.m. and p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the a.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the a.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday a.m. peak hour under future conditions for option 1-2. The only exception is the southbound right movement at the intersection of Washington Street and 1<sup>st</sup> Street, with a queue of 120ft, as compared to the storage length of 115ft.

#### 3.2.1.2 Option 1 – PM Traffic Capacity Analysis

The traffic capacity analysis for the p.m. peak hour for the three Option 1 configurations are shown in **Table 3-3** below.

**Table 3-3 Option 1-0 to 1-2 PM Traffic Capacity Analysis** 

		Storage	Optio	n 1-0 Wee	ekday PM our	l Peak	Optio	n 1-1 We Ho	ekday PM our	1 Peak	Optic	n 1-2 We H	ekday PM our	1 Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
						Signal	ized							
	Overall	-	0.32	15	В	-	0.32	15	В	-	0.32	15	В	-
	EBL	173	0.08	12	В	19	0.08	12	В	19	0.08	12	В	19
	EBTR	1	0.36	14	В	146	0.36	14	В	146	0.36	14	В	146
	WBL	98	0.08	12	В	23	0.08	12	В	23	0.08	12	В	23
1st St & Van Buren	WBTR	-	0.43	15	В	186	0.43	15	В	186	0.43	15	В	186
St	NBL	112	0.17	19	В	60	0.17	19	В	60	0.17	19	В	60
	NBT	-	0.13	18	В	68	0.13	18	В	68	0.13	18	В	68
	NBR	112	0.05	17	В	24	0.05	17	В	24	0.05	17	В	24
	SBL	95	0.10	18	В	37	0.10	18	В	37	0.10	18	В	37
	SBTR	-	0.10	18	В	47	0.10	18	В	47	0.10	18	В	47
	Overall	-	0.36	9	Α	-	0.36	9	Α	-	0.36	9	Α	-
	EBL	82	0.05	5	Α	9	0.05	5	Α	9	0.05	5	Α	9
	EBTR	-	0.35	7	Α	150	0.35	7	Α	150	0.35	7	Α	150
3rd St &	WBL	46	0.08	1	Α	2	0.08	1	Α	2	0.08	1	Α	2
Van Buren	WBTR	-	0.31	4	Α	161	0.31	4	Α	161	0.31	4	Α	161
St	NBL	151	0.07	31	С	17	0.07	31	С	17	0.07	31	С	17
	NBTR	-	0.06	31	С	30	0.06	31	С	30	0.06	31	С	30
	SBL	144	0.17	32	С	45	0.17	32	С	45	0.17	32	С	45
	SBTR	-	0.47	34	С	122	0.47	34	С	122	0.47	34	С	122
	Overall	-	0.18	20	С	-	0.18	20	С	-	0.18	20	С	-





		Storage	Option	n 1-0 Wee	ekday PM	l Peak	Optio	n 1-1 We Ho	ekday PM our	1 Peak	Optio	n 1-2 We H	ekday PM our	1 Peak
Intersection N	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	EBTR	-	0.08	4	Α	27	0.08	4	Α	27	0.08	4	Α	27
	WBL	158	0.03	3	Α	8	0.03	3	Α	8	0.03	3	Α	8
0.1010	WBT	-	0.05	3	Α	12	0.05	3	Α	12	0.05	3	Α	12
3rd St & Fillmore St	NBL	-	0.16	32	С	32	0.16	32	С	32	0.16	32	С	32
	NBTR	-	0.00	45	D	0	0.00	45	D	0	0.00	45	D	0
	SBL	-	0.08	31	С	32	0.08	31	С	32	0.08	31	С	32
	SBTR	-	0.61	37	D	167	0.61	37	D	167	0.61	37	D	167
	Overall	-	0.35	17	В	-	0.35	17	В	-	0.35	17	В	-
	EBL	-	0.25	6	Α	11	0.25	6	Α	11	0.25	6	Α	11
4th St &	EBT	-	0.71	14	В	39	0.71	14	В	39	0.71	14	В	39
Roosevelt	WBTR	98	0.42	28	С	119	0.42	28	С	119	0.42	28	С	119
St	NBL	-	0.07	12	В	48	0.07	12	В	48	0.07	12	В	48
	NBT	-	0.17	13	В	94	0.17	13	В	94	0.17	13	В	94
	NBR	-	0.00	11	В	0	0.00	11	В	0	0.00	11	В	0
	Overall	-	0.35	24	С	-	0.35	24	С	-	0.35	24	С	-
	EBTR	-	0.47	29	С	131	0.47	29	С	131	0.47	29	С	131
3rd St &	WBL	120	0.17	18	В	16	0.17	18	В	16	0.17	18	В	16
Roosevelt	WBT	-	0.79	30	С	84	0.79	30	С	84	0.79	30	С	84
St	NBL	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	NBR	-	0.00	0	0		0.00	0	0	-	0.00	0	0	-
	SBLTR		0.16	10	Α	75	0.16	10	Α	75	0.16	10	Α	75
	Overall	-	0.40	38	D	-	0.40	38	D	-	0.40	38	D	-
	WBL	190	0.17	18	В	68	0.17	18	В	68	0.17	18	В	68
3rd St & I10	WBR	-	0.06	17	В	28	0.06	17	В	28	0.06	17	В	28
HOV	NBTR	-	0.38	19	В	115	0.38	19	В	115	0.38	19	В	115
	SBL	121	1.12	132	F	267*	1.12	132	F	267*	1.12	132	F	267*
	SBT	-	0.30	18	В	100	0.30	18	В	100	0.30	18	В	100
	Overall	-	0.16	8	Α	-	0.00	43	D	-	0.00	43	D	-
	EBTR	-	2.20dl	1	Α	43	0.00	0	0	-	0.00	0	0	-
	WBL	105	0.01	1	Α	2	0.00	0	0	-	0.00	0	0	-
3rd St &	WBT	-	0.08	1	Α	18	0.00	0	0	-	0.00	0	0	-
Lincoln St	NBL	-	0.19	43	D	28	0.00	0	0	-	0.00	0	0	-
	NBTR	157	0.00	47	D	0	0.00	0	0	-	0.00	0	0	-
	SBL	144	0.25	43	D	41	0.00	0	0	-	0.00	0	0	-
	SBTR	-	0.29	43	D	56	0.07	43	D	0	0.07	43	D	0
5th St &	Overall	-	0.27	8	Α	-	0.27	8	Α	-	0.27	8	Α	-
Van Buren	EBL	131	0.12	5	Α	24	0.12	5	Α	24	0.12	5	Α	24





		Storage	Optio	n 1-0 Wee	ekday PM our	1 Peak	Optio	n 1-1 We Ho	ekday PN our	1 Peak	Optio	on 1-2 We H	ekday PN our	1 Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	EBTR	-	0.29	5	Α	115	0.29	5	Α	115	0.29	5	Α	115
	WBT	-	0.28	8	Α	112	0.28	8	Α	112	0.28	8	Α	112
	WBR	207	0.05	6	Α	14	0.05	6	Α	14	0.05	6	Α	14
	NBLTR	-	0.12	32	С	33	0.12	32	С	33	0.12	32	С	33
	Overall	-	0.20	23	С	-	0.20	23	С	-	0.20	23	С	-
5th St/4th	EBL	128	0.09	4	Α	15	0.09	4	Α	15	0.09	4	Α	15
St &	EBT	-	0.09	3	Α	16	0.09	3	Α	16	0.09	3	Α	16
Fillmore St	WBTR	-	0.06	4	Α	16	0.06	4	Α	16	0.06	4	Α	16
	NBLTR	-	0.64	36	D	112	0.64	36	D	112	0.64	36	D	112
	Overall	-	0.18	6	Α	-	0.18	6	Α	-	0.18	6	Α	-
5th St &	WBT	-	0.17	1	Α	18	0.17	1	Α	18	0.17	1	Α	18
Washington St	WBR	295	0.04	1	Α	m5	0.04	1	A	m5	0.04	1	Α	m5
	NBTL	-	0.26	54	D	40	0.26	54	D	40	0.26	54	D	40
5th St &	Overall	-	0.29	2	Α	-	0.29	2	A	-	0.29	2	Α	-
Jefferson	EBTL	-	0.29	2	Α	132	0.29	2	Α	140	0.29	2	Α	140
St	EBR	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.31	11	В	-	0.15	0	Α	-	0.20	0	Α	-
	EBTL	-	0.21	6	Α	61	0.12	0	Α	0	0.16	0	Α	0
1st St &	EBR	144	0.06	5	Α	28	0.00	0	0	-	-	-	-	-
Jefferson St	NBT	112	0.26	18	В	57	0.00	0	0	-	0.00	0	0	-
	NBR	-	0.38	19	В	50	0.00	0	0	-	0.00	0	0	-
	SBTL	-	0.57	22	С	92	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.34	16	В	-	0.35	13	В	-	0.57	34	С	-
	WBTL	-	0.23	10	Α	75	0.31	11	В	116	0.31	11	В	116
	WBR	161	0.04	8	Α	17	0.04	8	Α	17	0.04	8	Α	17
Washington St & 1st St	NBL	-	1	-	ı	-	-	-	-	-	0.44	25	С	146
01 01 101 01	NBTL	-	0.52	27	С	177	0.00	0	0	-	1.00	66	E	545*
	SBTR	-	0.22	21	С	92	-	-	-	-	-	-	-	-
	SBR	115	0.13	21	С	46	0.42	25	С	127	0.42	25	С	127
	Overall	-	0.27	27	С	-	0.25	29	С	-	0.25	29	С	-
3rd St &	WBTL	-	0.64	32	С	165	0.70	29	С	221	0.70	29	С	221
Washington	WBR	210	0.08	26	С	31	0.06	21	С	26	0.06	21	С	26
St	NBTL	-	0.00	0	-	-	0.00	0	-	-	0.00	0	-	-
	SBTR	-	0.13	6	Α	60	0.00	0	-	-	0.00	0	-	-
	Overall	-	0.31	26	С	-	0.31	26	С	-	0.31	26	С	-
	EBL	89	0.16	22	С	37	0.16	22	С	37	0.16	22	С	37





Ī		Storage	Optio	n 1-0 Wee	ekday PM our	l Peak	Optio	n 1-1 We	ekday PM our	1 Peak	Optio	n 1-2 We H	ekday PN our	/I Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	EBTR	-	0.71	33	С	327	0.71	33	С	327	0.71	33	С	327
	WBL	92	0.31	27	С	53	0.31	27	С	53	0.31	27	С	53
1st St &	WBTR	-	0.56	28	С	253	0.56	28	С	253	0.56	28	С	253
Roosevelt	NBL	115	0.04	9	Α	20	0.04	9	Α	20	0.04	9	Α	20
St	NBTR	-	0.07	9	Α	29	0.07	9	Α	29	0.07	9	Α	29
	SBL	112	0.04	9	Α	18	0.04	9	Α	18	0.04	9	Α	18
	SBTR	-	0.04	9	Α	18	0.04	9	Α	18	0.04	9	Α	18
	Overall	-	0.25	11	В	-	0.19	10	Α	-	0.19	10	Α	-
	EBT	-	0.29	10	Α	77	0.32	10	Α	85	0.32	10	Α	85
3rd St &	EBR	164	0.11	9	Α	38				-				-
Jefferson St	NBTR	-	0.00	0	0	-	0.00	0	0		0.00	0	0	-
	SBL	141	0.14	16	В	49	0.00	0	0		0.00	0	0	-
	SBT	-	0.20	16	В	71	0.00	0	0	-	0.00	0	0	-
•				<u>I</u>		Unsigna	alized					U.		
2.1	EBTR	-	0.15	8	Α	-	0.16	8	Α	-	0.16	8	Α	-
3rd Street &	WBLT	-	0.05	8	Α	-	0.05	8	Α	-	0.05	8	Α	-
Jackson	NBLTR	-	0.00	8	А	-	0.00	8	Α	-	0.00	8	Α	-
Street	SBLTR	-	0.22	8	Α	-	0.22	8	Α	-	0.22	8	Α	-
	EBLTR	-	0.28	10	A	-	0.27	9	Α	-	0.27	9	Α	-
1st Street	WBLTR	-	0.13	8	Α	•	0.14	9	Α	-	0.14	9	Α	-
&	NBLTR	-	0.17	9	Α	-	0.16	9	Α	-	0.16	9	Α	-
Jackson - Street	SBL	-	0.03	8	Α	-	0.03	8	Α	-	0.03	8	Α	-
	SBTR	-	0.18	8	-	-	0.18	-	-	-	0.18	8	-	-
	EBLT	-	0.03	2	Α	2	0.26	8	Α	26	0.26	8	Α	26
	EBTR	-	0.11	0	-	-	-	-	-	-	-	-	-	-
1st Street	WBLT	-	0.00	0	Α	-	0.00	0	Α	0	0.00	0	Α	0
& Lincoln Street	WBTR	-	0.09	0	-	-	0.09	0	-	0	0.09	0	-	0
Street	NBLTR	-	0.09	14	В	8	0.26	38	Е	25	0.26	38	Е	25
	SBLTR	-	0.32	16	С	33	0.62	41	Е	92	0.62	41	Е	92
1	EBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
1st Street &	WBLTR	89	-	-	-	-	-	-	-	-	-	-	-	-
Fillmore	NBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
Street				1		l		<b> </b>	<del> </del>	1		<del>                                     </del>		1

## <u>1-0 PM – Future conditions without any events</u>

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during





both the weekday p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a LOS of F. It should be noted that the movement was already experiencing LOS F during the existing p.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a volume to capacity ration of 1.12. It should be noted that the movement was already experiencing a volume to capacity ration of 1.12 during the existing p.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday p.m. peak hour under future conditions for option 1-0. The only exception is the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, with a queue of 267ft, as compared to the storage length of 121ft.

#### 1-1 PM – Future conditions with <25,000 Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a LOS of F. It should be noted that the movement was already experiencing LOS F during the existing p.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a volume to capacity ration of 1.12. It should be noted that the movement was already experiencing a volume to capacity ration of 1.12 during the existing p.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday p.m. peak hour under future conditions for option 1-1. The only exception is the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, with a queue of 267ft, as compared to the storage length of 121ft.

#### 1-2 PM – Future conditions with 25,000+ Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a LOS of F, and the





northbound through-left movement at the intersection of Washington Street and 1<sup>st</sup> Street, with LOS E. It should be noted that the movement at the intersection of I-10 and 3<sup>rd</sup> Street was already experiencing LOS F during the existing p.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, and the northbound through-left movement at the intersection of Washington Street and 1<sup>st</sup> Street. The intersection of I-10 and 3<sup>rd</sup> Street experienced a volume to capacity ration of 1.12, it should be noted that the movement was already experiencing the same ratio during the existing p.m. peak hour. The northbound through-left movement at Washington Street and 1<sup>st</sup> Street reached capacity at a ratio of 1.00 during the p.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday p.m. peak hour under future conditions for option 1-2. The only exception is the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, with a queue of 267ft, as compared to the storage length of 121ft.

#### 3.2.2 Capacity Analysis for Option 2

#### 3.2.2.1 Option 2 – AM Traffic Capacity Analysis

The traffic capacity analysis for the AM peak hour for the three Option 2 configurations are shown in **Table 3-4** below.

**Table 3-4** Option 2-0 to 2-2 AM Traffic Capacity Analysis

		Storage	Optic	n 2-0 We H	ekday AN our	/I Peak	Optio	on 2-1 We H	ekday AN our	/I Peak	Optio	on 2-2 We H	ekday Al our	M Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
						Signali	zed							
	Overall	-	0.30	8	Α	-	0.30	8	Α	-	0.30	8	Α	-
	EBL	173	0.03	3	Α	8	0.03	3	Α	8	0.03	3	Α	8
	EBTR	-	0.30	4	Α	75	0.30	4	Α	75	0.30	4	Α	75
	WBL	98	0.19	5	Α	28	0.19	5	Α	28	0.19	5	Α	28
1st St & Van	WBTR	-	0.22	4	Α	55	0.22	4	Α	55	0.22	4	Α	55
Buren St	NBL	112	0.27	28	С	48	0.27	28	С	48	0.27	28	С	48
	NBT	-	0.17	27	С	45	0.17	27	С	45	0.17	27	С	45
	NBR	112	0.02	27	С	18	0.02	27	С	18	0.02	27	С	18
	SBL	95	0.14	27	С	28	0.14	27	С	28	0.14	27	С	28
	SBTR	-	0.29	28	С	65	0.29	28	С	65	0.29	28	С	65
	Overall	-	0.27	11	В	-	0.29	10	Α	-	0.29	10	Α	-
i	EBT	-	0.29	7	Α	100	0.32	8	Α	108	0.32	8	Α	108





		Storage	Optio	on 2-0 We H	ekday AN our	/I Peak	Optio	on 2-1 We H	ekday AN	/I Peak	Optio	on 2-2 We H	ekday AN our	/I Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	EBR	-	0.00	38	D	0	-	-	-	-	-	-	-	-
3rd St & Van Buren	WBL	46	0.05	5	Α	9	0.05	5	Α	9	0.05	5	Α	9
St	WBT	-	0.26	7	Α	91	0.26	7	Α	91	0.26	7	Α	91
	SBLTR	-	0.24	25	С	54	0.24	25	С	54	0.24	25	С	54
	Overall	-	0.14	22	С	-	0.14	22	С	-	0.14	22	С	-
	EBTR	-	0.04	4	Α	14	0.04	4	Α	14	0.04	4	Α	14
3rd St & Fillmore St	WBL	158	0.02	3	Α	8	0.02	3	Α	8	0.02	3	Α	8
1 11111010 01	WBT	-	0.03	3	Α	11	0.03	3	Α	11	0.03	3	Α	11
	SBLTR	-	0.55	35	С	130	0.55	35	С	130	0.55	35	С	130
	Overall	-	0.18	18	В	-	0.18	18	В	-	0.18	18	В	-
	EBL	-	0.17	5	Α	7	0.17	5	Α	7	0.17	5	Α	7
4th St &	EBT	-	0.60	10	Α	31	0.60	10	Α	31	0.60	10	Α	31
Roosevelt	WBTR	98	0.39	31	С	98	0.39	31	С	98	0.39	31	С	98
St	NBL	-	0.02	8	Α	17	0.02	8	Α	17	0.02	8	Α	17
	NBT	-	0.04	8	Α	25	0.04	8	Α	25	0.04	8	Α	25
	NBR	-	0.00	8	Α	0	0.00	8	Α	0	0.00	8	Α	0
	Overall	-	0.27	18	В	-	0.27	18	В	-	0.27	18	В	-
3rd St &	EBTR	-	0.41	33	С	102	0.41	33	С	102	0.41	33	С	102
Roosevelt	WBL	120	0.15	11	В	9	0.15	11	В	9	0.15	11	В	9
St	WBT		0.66	17	В	41	0.66	17	В	41	0.66	17	В	41
	SBLTR	-	0.16	7	Α	68	0.16	7	Α	68	0.16	7	Α	68
	Overall	-	0.28	19	В	-	0.28	19	В	-	0.28	19	В	-
	WBL	190	0.30	19	В	110	0.30	19	В	110	0.30	19	В	110
3rd St & I10	WBR	-	0.14	18	В	44	0.14	18	В	44	0.14	18	В	44
HOV	NBTR	-	0.17	17	В	53	0.17	17	В	53	0.17	17	В	53
	SBL	121	0.41	36	D	82	0.41	36	D	82	0.41	36	D	82
	SBT	-	0.21	17	В	72	0.21	17	В	72	0.21	17	В	72
	Overall	-	0.14	33	С	-	0.03	0	Α	-	0.02	0	Α	-
	EBTR	-	0.45	34	С	106	0.00	0	0	-	0.00	0	0	-
	WBL	105	0.06	31	С	16	0.00	0	0	-	0.00	0	0	-
3rd St &	WBT	·	0.56	35	С	133	0.00	0	0	-	0.00	0	0	-
Lincoln St	NBL	-	0.00	4	Α	4	0.00	0	0	-	0.00	0	0	-
	NBTR	157	0.00	47	D	0	0.00	0	0	-	0.00	0	0	-
	SBL	144	0.01	4	А	10	0.00	0	0	-	0.02	0	А	-
	SBTR	-	0.02	4	А	9	0.03	0	Α	0	-	-	-	0
	Overall	-	0.24	7	Α	-	0.24	7	Α	-	0.24	7	Α	-
	EBL	131	0.12	3	Α	18	0.12	3	Α	18	0.12	3	Α	18





		Storage	Optio	on 2-0 We H	ekday AN	/I Peak	Optio	on 2-1 We He	ekday AN	/I Peak	Optio	on 2-2 We H	ekday AN	/I Peak
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	EBTR	-	0.21	3	Α	58	0.21	3	Α	58	0.21	3	Α	58
5th St &	WBT	-	0.26	6	Α	102	0.26	6	Α	102	0.26	6	Α	102
Van Buren St	WBR	207	0.12	6	Α	20	0.12	6	Α	20	0.12	6	Α	20
	NBLTR	-	0.11	34	С	24	0.11	34	С	24	0.11	34	С	24
	Overall	-	0.10	12	В	-	0.10	12	В	-	0.10	12	В	-
5th St/4th St	EBL	128	0.03	6	Α	16	0.03	6	Α	16	0.03	6	Α	16
& Fillmore	EBT	-	0.05	6	Α	26	0.05	6	Α	26	0.05	6	Α	26
St	WBTR	-	0.09	4	Α	25	0.09	4	Α	25	0.09	4	Α	25
	NBLTR	-	0.12	32	С	32	0.12	32	С	32	0.12	32	С	32
	Overall	-	0.24	5	Α	-	0.24	5	Α	-	0.24	5	Α	-
5th St &	WBT	-	0.25	2	Α	51	0.25	2	Α	51	0.25	2	Α	51
Washington St	WBR	295	0.04	2	Α	m11	0.04	2	A	m11	0.04	2	Α	m11
	NBTL	-	0.20	53	D	32	0.20	53	D	32	0.20	53	D	32
	Overall	-	0.09	1	Α	-	0.09	1	Α	-	0.09	1	Α	-
5th St & Jefferson St	EBTL	-	0.09	1	Α	40	0.10	1	Α	46	0.10	1	Α	46
Jellerson St	EBR	-	0.00	0	0		0.00	0	0	-	0.00	0	0	-
	Overall	-	0.15	12	В	-	0.07	0	Α	-	0.17	0	Α	-
	EBTL	-	0.08	3	Α	20	0.06	0	Α	0	0.14	0	Α	0
1st St &	EBR	144	0.04	3	Α	16	0.00	0	0	-	-	-	-	-
Jefferson St	NBT	112	0.47	24	С	80	0.00	0	0	-	0.00	0	0	-
	NBR	-	0.17	21	С	25	0.00	0	0	-	0.00	0	0	-
	SBTL		0.44	24	С	67	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.29	15	В	-	0.31	11	В	-	0.35	18	В	-
	WBTL	-	0.18	5	Α	64	0.24	5	Α	90	0.25	6	Α	103
	WBR	161	0.04	4	Α	15	0.04	4	Α	14	0.04	4	Α	17
Washington St & 1st St	NBL	-	-	-	-	-	-	-	-	-	0.11	28	С	44
Ot & 13t Ot	NBTL	-	0.70	39	D	155	0.00	0	0	-	0.69	36	D	194
	SBTR	-	0.25	29	С	72	-	-	-	-	-	-	-	-
	SBR	115	0.28	30	С	67	0.62	37	D	128	0.54	32	С	120
3rd St &	Overall	-	0.26	26	С	-	0.29	26	С	-	0.29	26	С	-
Washington	WBTL	-	0.63	29	С	188	0.70	26	С	258	0.70	26	С	258
St	SBTR	-	0.08	7	Α	37	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.19	10	В	-	0.19	10	В	-	0.19	10	В	-
1st St &	EBL	89	0.07	7	А	23	0.07	7	Α	23	0.07	7	Α	23
Roosevelt	EBTR	-	0.24	8	Α	98	0.24	8	А	98	0.24	8	А	98
St	WBL	92	0.08	7	Α	26	0.08	7	А	26	0.08	7	А	26
	WBTR	-	0.22	8	Α	93	0.22	8	Α	93	0.22	8	Α	93





		Storage	Optio	n 2-0 We H	ekday AN our		Optio	on 2-1 We H	ekday AN our		Optio	on 2-2 We H	ekday AN our	
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	NBL	115	0.05	24	С	26	0.05	24	С	26	0.05	24	С	26
	NBTR	-	0.03	23	С	25	0.03	23	С	25	0.03	23	С	25
	SBL	112	0.02	23	С	15	0.02	23	С	15	0.02	23	С	15
	SBTR	-	0.07	24	С	37	0.07	24	С	37	0.07	24	С	37
	Overall	-	0.09	9	Α	-	0.06	0	Α	-	0.06	0	Α	1
3rd St &	EBT	-	0.07	2	Α	14	0.06	0	Α	0	0.06	0	Α	0
Jefferson St	EBR	164	0.02	2	Α	7	-	-	-	-	-	-	-	-
	SBTL	-	0.30	28	С	41	0.00	0	0	-	0.00	0	0	-
						Unsigna	lized						•	
	EBTR	-	0.09	7	Α	-	0.06	7	Α	-	0.06	7	Α	ı
3rd Street	WBLT	-	0.03	7	Α	-	0.02	7	Α	-	0.03	7	Α	-
& Jackson Street	SBLT	-	0.04	7	Α	-	0.04	7	Α	-	0.04	7	Α	-
	SBTR	-	0.09	6	-	-	0.09	6	-	-	0.09	6	-	-
	EBLTR	-	0.10	8	Α	-	0.10	8	Α	-	0.10	8	Α	-
1st Street	WBLTR	-	0.14	8	Α		0.15	9	Α	-	0.15	9	Α	-
& Jackson	NBLTR	-	0.25	9	Α	-	0.25	9	Α	-	0.25	9	Α	-
Street	SBL	-	0.03	8	Α	-	0.03	8	Α	-	0.03	8	Α	-
	SBTR	-	0.09	7	-	-	0.09	7	-	-	0.09	7	-	-
	EBLT	-	0.06	3	Α	5	0.28	8	Α	29	0.28	8	Α	29
	EBTR		0.10	0	-	0	-	-	-	-	-	-	-	-
1st Street	WBLT		0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0
& Lincoln Street	WBTR	-	0.14	0	-	0	0.14	0	-	0	0.14	0	-	0
Street	NBLTR	-	0.08	15	С	7	0.22	35	D	20	0.22	35	D	20
	SBLTR	-	0.10	13	В	8	0.21	24	С	19	0.21	24	С	19
1 ct Ctro ct	EBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
1st Street &	WBLTR	89	-	-	-	-	-	-	-	-	-	-	-	-
Fillmore	NBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
Street	SBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-

#### 2-0 AM – Future conditions without any events

Under future conditions considering traffic in only the south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday a.m. peak hours. Individual movements and over intersections operate with LOS D or better during the a.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range





during the a.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday a.m. peak hour under future conditions for option 2-0.

#### 2-1 AM – Future conditions with <25,000 Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday a.m. and p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the a.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the a.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday a.m. peak hour under future conditions for option 2-1.

#### 2-2 AM - Future conditions with 25,000+ Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday a.m. and p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the a.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the a.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday a.m. peak hour under future conditions for option 2-2.

#### 3.2.2.2 Option 2 – PM Traffic Capacity Analysis

The traffic capacity analysis for the PM peak hour for the three Option 2 configurations are shown in **Table 3-5** below.

**Table 3-5 Option 2-0 to 2-2 PM Traffic Capacity Analysis** 

		Storage	Optio	n 2-0 We H	ekday PN our	/I Peak	Optio	n 2-1 We H	ekday PN our	/I Peak	Option	n 2-2 Weekda	y PM Pe	ak Hour
Intersection	Movement	Length (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)	v/c	Delay (s)	LOS	95% Queue (ft)
	Signalized													
	Overall	-	0.32	15	В	-	0.32	15	В	-	0.32	15	В	-
1st St &	EBL	173	0.08	12	В	19	0.08	12	В	19	0.08	12	В	19
Van Buren St	EBTR	-	0.36	14	В	146	0.36	14	В	146	0.36	14	В	146
	WBL	98	0.08	12	В	23	0.08	12	В	23	0.08	12	В	23





	WBTR	-	0.43	15	В	186	0.43	15	В	186	0.43	15	В	186
	NBL	112	0.17	19	В	60	0.17	19	В	60	0.17	19	В	60
	NBT	-	0.13	18	В	68	0.13	18	В	68	0.13	18	В	68
	NBR	112	0.05	17	В	24	0.05	17	В	24	0.05	17	В	24
	SBL	95	0.10	18	В	37	0.10	18	В	37	0.10	18	В	37
	SBTR	-	0.10	18	В	47	0.10	18	В	47	0.10	18	В	47
	Overall	-	0.32	9	Α	-	0.33	9	Α	-	0.33	9	Α	-
	EBT	-	0.34	7	Α	137	0.35	7	Α	141	0.35	7	Α	141
3rd St &	EBR	-	0.00	45	D	0	-	-	-	-	-	-	-	-
Van Buren St	WBL	46	0.08	1	Α	20	0.08	1	Α	2	0.08	1	Α	2
	WBT	-	0.30	4	Α	149	0.30	4	Α	149	0.30	4	Α	149
	SBLTR	-	0.30	33	С	68	0.30	33	С	68	0.30	33	С	68
	Overall	-	0.13	15	В	-	0.13	15	В	-	0.13	#VALUE!	F	-
	EBTR	-	0.08	4	Α	22	0.08	4	Α	22	0.08	4	Α	22
3rd St & Fillmore St	WBL	158	0.03	2	Α	6	0.03	2	Α	6	0.03	2	Α	6
Tillinore ot	WBT	-	0.05	2	Α	9	0.05	2	Α	9	0.05	2	Α	9
	SBLTR	-	0.36	33	С	89	0.36	33	С	89	0.36	33	С	89
	Overall	-	0.35	17	В	-	0.35	17	В	-	0.35	17	В	-
	EBL	-	0.25	6	Α	11	0.25	6	Α	11	0.25	6	Α	11
4th St &	EBT	-	0.71	14	В	39	0.71	14	В	39	0.71	14	В	39
Roosevelt	WBTR	98	0.42	28	С	119	0.42	28	С	119	0.42	28	С	119
St	NBL	-	0.07	12	В	48	0.07	12	В	48	0.07	12	В	48
	NBT		0.17	13	В	94	0.17	13	В	94	0.17	13	В	94
	NBR	-	0.00	11	В	0	0.00	11	В	0	0.00	11	В	0
	Overall	-	0.35	24	С	-	0.35	24	С	-	0.35	24	С	-
3rd St &	EBTR	-	0.47	29	С	131	0.47	29	С	131	0.47	29	С	131
Roosevelt	WBL	120	0.17	18	В	16	0.17	18	В	16	0.17	18	В	16
St	WBT	-	0.79	30	С	84	0.79	30	С	84	0.79	30	С	84
	SBLTR	-	0.16	10	Α	75	0.16	10	Α	75	0.16	10	Α	75
	Overall	-	0.40	38	D	-	0.40	38	D	-	0.40	38	D	-
	WBL	190	0.17	18	В	68	0.17	18	В	68	0.17	18	В	68
3rd St & I10	WBR	-	0.06	17	В	28	0.06	17	В	28	0.06	17	В	28
HOV	NBTR	-	0.38	19	В	115	0.38	19	В	115	0.38	19	В	115
	SBL	121	1.12	132	F	267*	1.12	132	F	267*	1.12	132	F	267*
	SBT	-	0.30	18	В	100	0.30	18	В	100	0.30	18	В	100
	Overall	-	0.15	8	Α	-	0.00	43	D	-	0.00	43	D	-
	EBTR	-	0.14	1	Α	33	0.00	0	0	-	0.00	0	0	-
3rd St & Lincoln St	WBL	105	0.01	1	Α	2	0.00	0	0	-	0.00	0	0	-
LINGUIT OL	WBT	-	0.08	1	Α	19	0.00	0	0	-	0.00	0	0	-
	NBL	-	0.19	43	D	28	0.00	0	0	-	0.00	0	0	-





	NBTR	157	0.00	47	D	0	0.00	0	0	_	0.00	0	0	-
	SBL	144	0.25	43	D	41	0.00	0	0	-	0.03	43	D	-
	SBTR	-	0.29	43	D	56	0.07	43	D	0	-	-	-	0
	Overall	-	0.27	8	Α	-	0.27	8	Α	-	0.27	8	Α	-
	EBL	131	0.12	4	Α	18	0.12	4	Α	18	0.12	4	Α	18
5th St &	EBTR	-	0.29	4	Α	83	0.29	4	Α	83	0.29	4	Α	83
Van Buren St	WBT	-	0.28	8	Α	112	0.28	8	Α	112	0.28	8	Α	112
	WBR	207	0.05	6	Α	14	0.05	6	Α	14	0.05	6	Α	14
	NBLTR	-	0.12	32	С	33	0.12	32	С	33	0.12	32	С	33
	Overall	-	0.20	23	С	-	0.20	23	С	-	0.20	23	С	-
5th St/4th	EBL	128	0.09	4	Α	29	0.09	4	Α	29	0.09	4	Α	m20
St &	EBT	-	0.09	3	Α	20	0.09	3	Α	20	0.09	3	Α	m15
Fillmore St	WBTR	-	0.06	4	Α	16	0.06	4	Α	16	0.06	4	Α	16
	NBLTR	-	0.64	36	D	112	0.64	36	D	112	0.64	36	D	112
	Overall	-	0.18	6	Α	-	0.18	6	Α	-	0.18	6	Α	-
5th St &	WBT	-	0.17	1	Α	18	0.17	1	Α	18	0.17	1	Α	18
Washington St	WBR	295	0.04	1	Α	m5	0.04	1	Α	m5	0.04	1	Α	m5
	NBTL	-	0.26	54	D	40	0.26	54	D	40	0.26	54	D	40
	Overall	-	0.29	2	Α	-	0.29	2	Α	-	0.29	2	Α	-
5th St & Jefferson St	EBTL	-	0.29	2	Α	132	0.29	2	Α	140	0.29	2	Α	140
001101001100	EBR	-	0.00	0	0	-	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.31	11	В	-	0.15	0	Α	-	0.41	0	Α	-
	EBTL	-	0.21	6	Α	61	0.12	0	Α	0	0.34	0	Α	0
1st St &	EBR	144	0.06	5	Α	28	0.00	0	0	-	-	-	-	-
Jefferson St	NBT	112	0.26	18	В	57	0.00	0	0	-	0.00	0	0	-
	NBR	-	0.38	19	В	50	0.00	0	0	-	0.00	0	0	-
	SBTL	-	0.57	22	С	92	0.00	0	0	-	0.00	0	0	-
	Overall	-	0.34	16	В	-	0.35	13	В	-	0.57	34	С	-
	WBTL	-	0.23	10	Α	75	0.31	11	В	116	0.31	11	В	116
107 1 1	WBR	161	0.04	8	Α	17	0.04	8	Α	17	0.04	8	Α	17
Washington St & 1st St	NBL	-	-	-	-	-	-	-	-	-	0.44	25	С	146
	NBTL	-	0.52	27	С	177	0.00	0	0	-	1.00	66	Е	545*
	SBTR	-	0.22	21	С	92	-	-	-	-	-	-	-	-
	SBR	115	0.13	21	С	46	0.42	25	С	127	0.42	25	С	127
3rd St &	Overall	-	0.22	27	С	-	0.25	29	С	-	0.25	29	С	-
Washington	WBTL	-	0.63	31	С	164	0.70	29	С	224	0.70	29	С	224
St	SBTR	-	0.07	6	Α	28	0.00	0	0	-	0.00	0	0	-
1st St &	Overall	-	0.31	26	С	-	0.31	26	С	-	0.31	26	С	-
Roosevelt	EBL	89	0.16	22	С	37	0.16	22	С	37	0.16	22	С	37
St	EBTR	-	0.71	33	С	327	0.71	33	С	327	0.71	33	С	327





	WBL	92	0.31	27	С	53	0.31	27	С	53	0.31	27	С	53
	WBTR	-	0.56	28	С	253	0.56	28	С	253	0.56	28	С	253
	NBL	115	0.04	9	Α	20	0.04	9	Α	20	0.04	9	Α	20
	NBTR	-	0.07	9	Α	29	0.07	9	Α	29	0.07	9	Α	29
	SBL	112	0.04	9	Α	18	0.04	9	Α	18	0.04	9	Α	18
	SBTR	-	0.04	9	Α	18	0.04	9	Α	18	0.04	9	Α	18
	Overall	-	0.23	10	В	-	0.19	10	Α	-	0.19	10	Α	-
3rd St &	EBT	-	0.29	10	Α	77	0.32	10	Α	85	0.32	10	Α	85
Jefferson St	EBR	164	0.11	9	Α	38	-	-	-	-	-	-	-	-
	SBTL	-	0.14	15	В	40	0.00	0	0	-	0.00	0	0	-
						Unsign	nalized							
	EBTR	-	0.07	7	Α	-	0.16	8	Α	-	0.16	8	Α	-
3rd Street	WBLT	-	0.03	7	Α	-	0.05	8	Α	-	0.05	8	Α	-
& Jackson Street	SBLT		0.03	7	Α	-	0.11	8	Α	-	0.11	8	Α	-
Street	SBTR	-	0.09	6	-	-	0.15	7		-	0.15	7	-	-
	EBLTR	-	0.28	10	Α	-	0.27	9	Α	-	0.27	9	Α	-
1st Street	WBLTR	-	0.13	8	Α	-	0.14	9	Α	-	0.14	9	Α	-
& Jackson	NBLTR	-	0.17	9	Α	-	0.16	9	Α	-	0.16	9	Α	-
Street	SBL	-	0.03	8	Α	- 1	0.03	8	Α	-	0.03	8	Α	-
	SBTR	-	0.18	8		-	0.18	8	-	-	0.18	8	-	-
	EBLT	-	0.03	2	Α	2	0.26	8	Α	26	0.26	8	Α	26
	EBTR	-	0.11	0		0	-	-	-	-	-	-	-	-
1st Street	WBLT	-	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0
& Lincoln Street	WBTR	-	0.09	0	-	0	0.09	0	-	0	0.09	0	-	0
Street	NBLTR		0.09	14	В	8	0.26	38	Е	25	0.26	38	Е	25
	SBLTR	-	0.32	16	С	33	0.62	41	Е	92	0.62	41	Е	92
1st Street	EBLTR	98	-	-	-	-	-	-	-	-	-	-	-	-
1st Street &	WBLTR	89	-	-	-	-	-	-	-	-	-	-	-	-
Fillmore	NBLTR	98	-	-	-	-	-	-	-	-	ı	-	-	-
Street	SBLTR	98	-	1	-	-	-	1	-	-	i	-	-	-

#### 2-0 PM – Future conditions without any events

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a LOS of F. It should be noted that the movement was already experiencing LOS F during the existing p.m. peak hour.





Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a volume to capacity ration of 1.12. It should be noted that the movement was already experiencing a volume to capacity ration of 1.12 during the existing p.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday p.m. peak hour under future conditions for option 2-0. The only exception is the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, with a queue of 267ft, as compared to the storage length of 121ft.

#### 2-1 PM - Future conditions with <25,000 Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a LOS of F. It should be noted that the movement was already experiencing LOS F during the existing p.m. peak hour.

Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a volume to capacity ration of 1.12. It should be noted that the movement was already experiencing a volume to capacity ration of 1.12 during the existing p.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday p.m. peak hour under future conditions for option 2-1. The only exceptions were the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, with a queue of 267ft, as compared to the storage length of 121ft, and the southbound right movement for the intersection of Washington Street and 1<sup>st</sup> Street with a queue of 127ft, as compared to the storage length of 115ft.

#### 2-2 PM - Future conditions with 25,000+ Event

Under future conditions considering traffic in both the north and south direction along N 3<sup>rd</sup> Street, all study area intersections are operation with overall reserve capacity and acceptable delays during both the weekday p.m. peak hours. Individual movements and over intersections operate with LOS D or better during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, i.e., movements to the freeway ramp, with a LOS of F, and the northbound through-left movement at the intersection of Washington Street and 1<sup>st</sup> Street, with LOS E. It should be noted that the movement at the intersection of I-10 and 3<sup>rd</sup> Street was already experiencing LOS F during the existing p.m. peak hour.





Volume-to-capacity ratios at all study intersections and movements are within the acceptable range during the p.m. peak hour, with the exception of the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, and the northbound through-left movement at the intersection of Washington Street and 1<sup>st</sup> Street. The intersection of I-10 and 3<sup>rd</sup> Street experienced a volume to capacity ration of 1.12, it should be noted that the movement was already experiencing the same ratio during the existing p.m. peak hour. The northbound through-left movement at Washington Street and 1<sup>st</sup> Street reached capacity at a ratio of 1.00 during the p.m. peak hour.

Overall, study area intersections operate with acceptable 95<sup>th</sup> percentile queues during the weekday p.m. peak hour under future conditions for option 2-2. The only exceptions were the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, with a queue of 267ft, as compared to the storage length of 121ft, and the southbound right movement for the intersection of Washington Street and 1<sup>st</sup> Street with a queue of 127ft, as compared to the storage length of 115ft.

## 3.3 Mobility Concerns Raised in Public Engagement

During the course of this study, members of the public raised particular concerns regarding traffic operations at the intersection of 2<sup>nd</sup> Street and Taylor Street, within the Arizona State University campus. In particular, one user of this intersection, who frequently parks at the 200 EVB parking garage accessed from 2<sup>nd</sup> Street immediately south of the intersection, noted that high pedestrian crossing volumes at the existing all-way stop configuration sometimes result in excessively long wait times for vehicles to proceed. An image of this intersection, illustrating pedestrians crossing the intersection, is provided in **Figure 3-4.** 

The 2<sup>nd</sup> Street/Taylor Street intersection is located within the Arizona State University Downtown Campus, just to the east of the Taylor Street pedestrian mall between 1<sup>st</sup> Avenue and 1<sup>st</sup> Street, which logically may result in higher pedestrian volumes than in other areas of the study area road network. While it was not assessed as part of the traffic operations analysis in this study and therefore no turning movement count or pedestrian count was available for this intersection, making it difficult to verify these concerns or assess the feasibility of alternate modes of intersection control.

However, conceptually, signalization of the intersection and the addition of a pedestrian scramble signal, which is the existing configuration at the neighboring intersections of 1<sup>st</sup> Street/Taylor Street and 3<sup>rd</sup> Street/Taylor Street, may assist in better managing high volumes of pedestrians than the existing all-way stop control. Pedestrian compliance with signals, particularly in a university context, may however be an issue. Further study, including the collection of turning movement counts and active transportation counts, may however be necessary to assess whether signalization is warranted.

Driver education on possible detours to the 200 EVB parking garage (such as via Van Buren and 2<sup>nd</sup> Streets) to avoid the 2<sup>nd</sup> Street/Taylor Street intersection may also be beneficial.





Figure 3-4 2<sup>nd</sup> Street/Taylor Street Intersection



Source: Google Street View, November 2022 (facing north)





## 4 CONCLUSIONS AND RECOMMENDATIONS

The existing conditions traffic analysis of the study area road network reveals that the network broadly holds sufficient capacity to support any of the three alternative alignments proposed for the north-south active transportation corridor. Areas in which operational issues were apparent are rare and isolated within the network, and point to operational issues with individual turning movements as a cause rather than a broad inability of the network to handle the capacity of vehicular traffic moving through downtown Phoenix. Furthermore, the isolated areas in which operational issues were present did not tend to involve traffic movements which would be impacted by the implementation of the proposed active transportation corridor alignments; in one such case (the I-10 and 3<sup>rd</sup> Street intersection), the intersection has seen reconstruction and implementation of active transportation infrastructure since the time that turning movement counts were collected, regardless.

In the Future Traffic Analysis, considering both configurations of N 3<sup>rd</sup> Street, similar to the existing conditions, only the southbound left movement at the intersection of I-10 and 3<sup>rd</sup> Street, exceeded capacity during the p.m. peak hour.

Considering the Traffic control plan lane configurations for the future scenarios, there is sufficient capacity to support both configurations of a north-south, and only a southbound corridor along N 3<sup>rd</sup> Street. The only difference is that for southbound only traffic at the intersection of Washington Street and 1<sup>st</sup> Street, for events less than 25,000 the southbound right movement queue that exceeds capacity.

Road network capacity should therefore not be considered a limiting factor in the assessment of the proposed alternative alignments for the active transportation corridor. Any of the three alignments for this downtown corridor would be supported from a traffic perspective.

Implementation of a continuous north-south cycling connection through downtown Phoenix where no such continuous corridor currently exists may further yield benefits as a transportation demand management measure, encouraging cycling as a multi-modal alternative to the private automobile within this area.





## **ATTACHMENT A: SYNCHRO SUMMARY REPORTS**

# Attachment Content Available Upon Request

